Measuring deformation of bridge structures using laser scanning data

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ABSTRACT

Increasing traffic weights and aggressive environmental conditions may result in unexpected deterioration of a bridge's components. Particularly, most bridges in Europe and US over half life span are affected by such impact. Structural deficiencies may cause partial or full collapse of bridges resulting in problems for human life, economy, society and environment. As such, deformation measurement of the bridge's structural components has high priority in bridge inspection and assessment. Laser scanning has been used to capture the three-dimensional (3D) topographic surface of structures accurately and efficiently, which can be subsequently used to measure change of the structures. This paper introduces three approaches called point-to-surface (P2S), point-to-cell (P2C) and cell-to-cell (C2C) to measure the deformation of a structure using laser scanning data. This study also investigates the impact of the selected reference surface or cell size to the achieved accuracy of deformation measurement, which will be demonstrated through an implementation to measure the bridge's vertical clearance, which is the maximum vertical drop distance from the bottom of the bridge deck to the ground or water level.
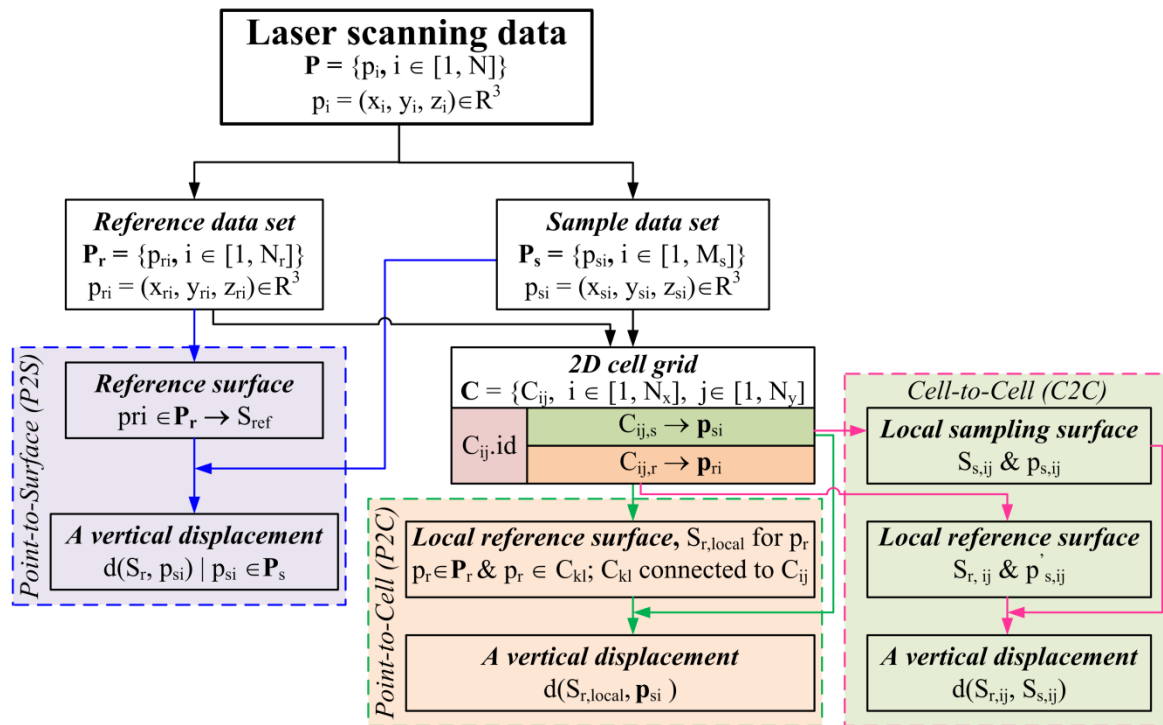
Figure 1: Methods for vertical displacement estimation from a point cloud

I. INTRODUCTION

With a designed 50-year lifespan, most bridges in US and Europe is subject to structural deficiencies (ASCE 2017; Pakrashi et al. 2011) because of excessive usage, overloading, material aging, and environmental impacts. As such, changes in a bridge structural components should be timely reported for maintaining a safe, functional, and reliable structure. The impact of such changes to structural integrity may manifest as alterations in the condition of the connections, deformations, distortions or embedment loss. Current bridge inspection procedures mainly rely on visual inspection with physical inspectors associated with special equipment on site, which has several downsides: (1) subjective results; (2) slow and expensive procedure; (3) high risk for inspectors and (4) traffic closures (Metni et al., 2007; Phares et al. 2004). Laser scanning is emerging as an alternative method to collect data for the bridge inspection, as

this technology can capture the current geometry of the structures accurately and efficiently. That is particularly compatible for measuring changes or deformation of structures (Lindenbergh et al., 2015). Thus, this paper focusses on processing laser scanning data (obtained by terrestrial or mobile laser scanning) to measure structural deformation, and as a case study, it is discussed and shown how vertical clearance estimated are obtained.

II. RELATED WORK

In the last decade, several methods have been proposed to identify and quantify changes in repeated point clouds acquired by aerial and terrestrial laser scanning (Mukupa et al., 2016). For example, Girardeau-Montaut et al. (2005) proposed a cloud to cloud (C2C) method to identify and quantify deformation through the distance from a point in a reference surface to its nearest neighbour point in a sampling surface. Additionally, to improve upon the accuracy of the C2C method, least squares was used to estimate a local surface through neighbouring points of a query point in the sampling point cloud, resulting in cloud to model (C2M) distances. Lague et al., (2013) proposed the so-called Multiscale Model to Model Cloud Comparison (M3C2) approach. In this method, a normal vector is first estimated for each point in the reference surface from its neighbouring points. Next, at a given point of the reference surface, a cylinder with a predefined radius in the direction of the normal vector of the point is established to extract sub-points of the reference and sampling surface. The local distance between two surfaces is defined as the distance between the average positions of two sub-clouds. The above methods are efficient in determining change detection in massive data rather than giving highly accurate results, as the accuracy depends on the normal vector of the points which is subject to point density and the noise level of the data set (Girardeau-Montaut et al., 2005; Lague et al., 2013). Particularly, those methods are mainly used for topographic change detection. This section is restricted to methods for structural deformation measurements, particularly for a bridge structure, while a systematic overview of the application of TLSs for bridge engineering has been published elsewhere [e.g. (Truong-Hong et al., 2014)].

Laser scanning has been used to measure overall displacements of a bridge (Lichti et al. 2002; Lovas et al., 2008; Zogg et al., 2008), vertical clearance (Riveiro et al., 2013; Liu et al., 2012), and deformations/distortions of each member (Truong-Hong et al., 2015). A deformation describes change of the surface in different instants (e.g. t_0 and t_1). For example, in structural engineering, deformation is defined as the distance between a surface at an epoch i (known as an undeformed or reference surface) and a surface at an epoch j (a deformed or sampling surface). In those applications, Kretschmer et al. (2004) and Truong-Hong et al. (2014) measure structural changes (vertical clearance and displacement) through a distance from a point of the sample surface to its projection on the reference surface. The projection was done based on a normal vector of a local reference surface determined from local neighbor points of the projection points derived from the reference surface. In another data processing direction, Lichti et al. (2002) measure vertical displacement of wood stringers by comparing fitting lines of the top and bottom stringers subject to unloaded and loaded conditions. Similarly, Riveiro et al. (2013) fitted a point cloud of a beam camber and pavement by a polynomial curve and a plane, respectively. Then, the vertical clearance of a bridge was as difference of z values computed from the curve and the plane. Finally, Paffenholz et al. (2008) subdivided a point cloud into 2D cell-grids with a cell size of 0.25m, and used the median of z coordinates of each cell to determine vertical displacements. Truong-Hong et al. (2015) also used a cell-based approach to measure vertical displacements of a beam as the distance between average z coordinates of the points within the cell to a reference surface.

III. METHODOLOGY

As laser scanning typically captures massive topographic data sampling a structures' surface, estimating structural deformation based on a point-to-point assessment is time consuming and impractical. In addition, of the presence of noise and/or mixed pixels also affect the estimation quality, particularly when millimeter accuracy is required. To address such issues, three methods are presented in this paper: point-to-surface (P2S), point-to-cell (P2C), and cell-to-cell (C2C), for which the workflows are shown in Figure 1. In addition, from a point of view of structural analysis, deformation of a structure involves directional and total deformation. However, to determine total deformation at a specific location in a structure directly from laser scanning data may be impractical: the structures surface should be captured at identical locations in different scans and epochs while extracting points representing the same location on the structure in different epochs in case of massive data may well be impossible. As such, the deformation mentioned here is a vertical deformation or displacement.

After scanning and registering all point clouds into a single coordinate system, input point clouds of interest are classified into reference and sampling data sets (P_r and P_s), respectively, describing the structures at different moment (or epochs). P_r represents the structure's surface (called a reference surface, S_r), which is often subjected to small deformation or easily

to identify a close-form formulation of the surface. P_s describes the structure surface (called a sampling surface, S_s) subject to large deformation. In the P2S approach, the shape of S_r is theoretically known a priori, and a close form of S_r is defined by an optimal fitting surface as expressed in Eq. 1. However, if the close form is not available, an optimization should be applied to identify the best fitting surface, in which the root mean squares error (RMSE) can be an indicator. In this approach, the directional (or vertical) deformation at a specific location on the structure is a distance from the data points $p_{si} \in P_s$ to S_r, which is given in Eq. 2.

$$S_r = f(x, y; \beta): \underset{\beta}{argmin}(\sum(z_i - f(x_i, y_i; \beta)^2) \quad (1)$$

where S_r = a formulation of the reference surface

(x_i, y_i, z_i) = coordinates of $p_{ri} \in P_r$

$$d(S_r, p_{si}) = |z_i - f(x_i, y_i; \beta)| \quad (2)$$

where $d(S_r, p_{si})$ = a distance from p_{si} to S_r

p_{si} = a point cloud of S_r

(x_i, y_i, z_i) = coordinates of $p_{si} \in P_s$

In the P2C and C2C methods, a 2D cell grid is employed to divide a bounded, 2D region of the data sets into a set of uniform cells. The process started to initially project the data set onto a plane of interest (PoI), for example an xy plane in a global coordinate system. Each cell is represented by an index C_{ij}, where $i \in [1, N_x]$ and $j \in [1, N_y]$, where N_x and N_y are the number of cells along x and y direction, as expressed in Eq.s 3 and 4. Each cell C_{ij} has two lists for indexing P_r and P_s, as notated by $C_{ij,r}$ and $C_{ij,s}$, respectively. This data management allows easy retrieval of the points ($p_{ij,r}$ and $p_{ij,s}$) in C_{ij} from P_r and P_s.

$$N_x = \left\lceil \frac{(x_{max} - x_{min})}{\Delta x} \right\rceil \quad (3)$$

$$N_y = \left\lceil \frac{(y_{max} - y_{min})}{\Delta y} \right\rceil \quad (4)$$

where $[x_{min}, y_{min}]$ = a minimum x and y coordinates

$[x_{max}, y_{max}]$ = a maximum x and y coordinates

Δx = a cell size along x direction

Δy = a cell size along y direction

In the P2C method, the vertical deformation is defined as the distance from a point $p_{si} \in P_s$ to an intersection point between a vertical ray through p_{si} and S_r (Truong-Hong et al., 2015). This assumption is based on the observation that the structural deformation between epochs is often small, while lateral displacement can be neglected. In this method, a local planar surface $S_{r,local}$ is used instead of S_r, which is estimated as follows: (1) from the cell C_{ij} containing the point p_{si}, a set of cells C_{kl} (k = [i-1, i+1] and l = [j-1, j+1] is extracted (Fig. 2a and b); (2) $p^r \in P_r$ are extracted from C_{kl}. Then, a robust principal component

analysis (PCA) (Laefer et al., 2017) is employed to determine the surface normal n = (n_x, n_y, n_z) of $S_{r,local}$ (Fig. 2c) from the covariance matrix C, as expressed in Eq. 5 and 6. Finally, vertical displacements are determined using Eq. 2.

$$C = \sum_{i=1}^{N}(p_i^r - p_0)(p_i^r - p_0)^T \quad (5)$$

$$S_{r,local} = n_x x + n_y y + n_z z + d \quad (6)$$

where p_0 (x_0, y_0, z_0) = a centroid of p_i

d = $-n_x x_0 - n_y y_0 - n_z y_0$

N = the number of data points

a) Point clouds within a cell

b) Extract $p^r \in P_r$ within C_{kl}

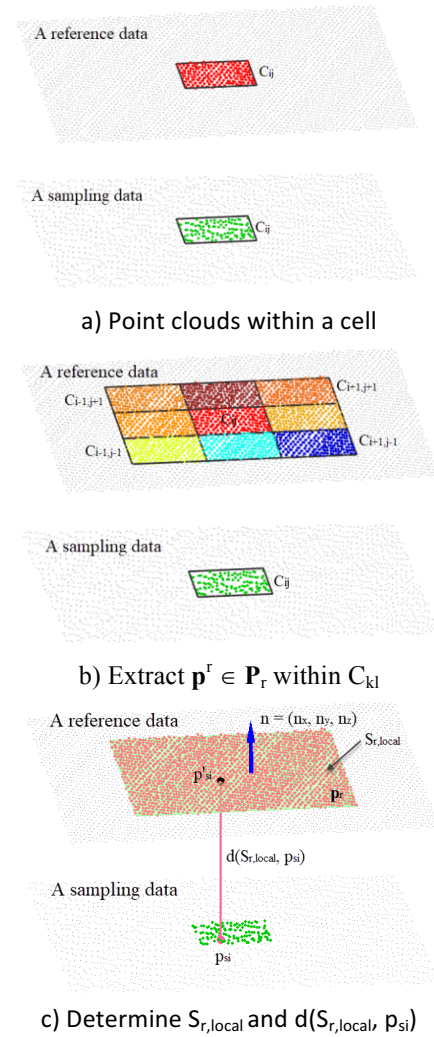c) Determine $S_{r,local}$ and $d(S_{r,local}, p_{si})$

Figure 2. Determining vertical deformation using the P2C method

In the C2C method, it is assumed that S_r and S_s at the specific location are respectively represented by the local planar surfaces $S_{r,ij}$ and $S_{s,ij}$. This implies that S_r and S_s can be represented by local planar surfaces at cells in the cell grid. Thus, for each cell C_{ij}, the local planar surfaces, $S_{r,ij}$ and $S_{s,ij}$ are respectively estimated from $p_{s,ij} \in P_s$ and $p_{r,ij} \in P_r$ within C_{ij} using PCA, as expressed in Eq. 5 and 6 (Fig. 3). The vertical deformation is herein defined as the directional distance between the centroid point of $p_{s,ij}$ on $S_{s,ij}$ to an intersection point (called $p'_{s,ij}$) between a vertical ray

through p_{si} and $S_{r,ij}$, as given in Eq. 2.
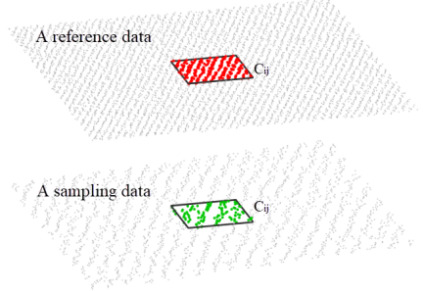
a) Point clouds within a cell
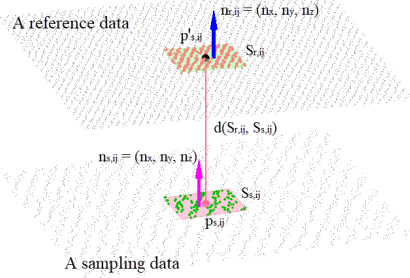
b) Estimate $S_{r,ij}$ and $S_{s,ij}$ and compute $d(S_{r,ij}, S_{s,ij})$

Figure 3. Determining vertical deformation using the C2C method

IV. CASE STUDY

The proposed methods are demonstrated by measuring the vertical clearance of an overpass bridge at the intersection between N25 and Coolballow Rd., Co. Wexford, Ireland (Fig. 4a). Data for the bridge and road were acquired by a Leica P20 TLS unit with a sampling step of 3.1 mm at a range measurement of 10 m (Fig. 4b). The average distance from the scanner to the bridge deck was around 15m. An octree-based region-growing approach (Vo et al., 2015) was employed to extract the point clouds of the bottom fibers of bridge's girders and of the road (Fig. 4c), which were considered as the reference and sampling data sets, respectively. As the vertical clearance is a goal of this example, only the point clouds of the road surface and the bottom fibers of the girders within the intersection of the convex hulls of the two data sets were used. Moreover, the point cloud of the bottom girder was separated using a clustering algorithm. For this purpose DBScan (Ester et al., 1996) was used (Fig. 4d), for which the input parameters, the maximum distance between two points (ε) and the minimum number of points (minPts) were selected as 0.4m and 20 points, respectively.
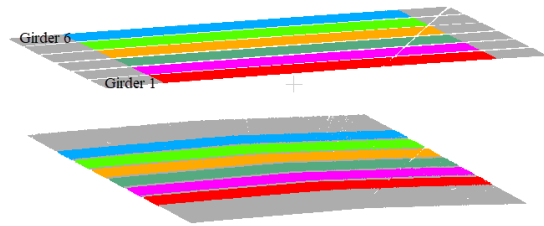

a) Photo of the bridge
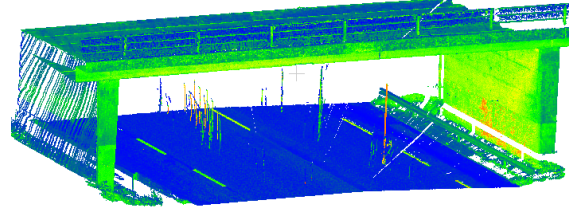
b) Point clouds of the bridge and road
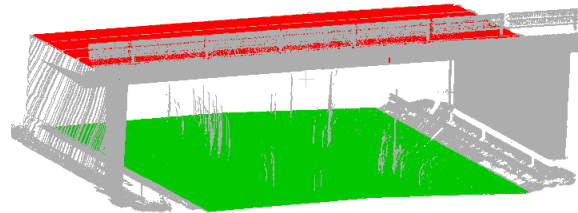
c) Point clouds of bottom surfaces of girders and road surface

d) Point clouds of bottom surfaces of each girder and its corresponding part at the road surface

Figure 4. Extracting point clouds of the bottom surfaces of the bridge's girders and road surface

For applying the P2S method, a fitting 3D surface was employed to fit the bottom fibers of the girders or the reference surface. However, depending on the design and construction of the bridge, some scenarios may apply: (1) the elevation of each girder may be different; (2) the bottom surface of the girder is unknown, which may be a planar or parabolic surface due to a camber or deflection of the girder. As such, different types of surfaces (Table 1) were proposed to fit the reference surface for each girder separately and the best fit model was determined based on the minimum RMSE (Table 2). Note that to prevent over-fitting, about 70% of random points of the reference surface were used to predict the fitting surface while the remaining points were used to validate the fitting model. Based on the RMSE, the fitting surface model, S_{r4} was chosen, and coefficients of the fitting model for each beam are shown in Table 3. Finally, the vertical clearance values were computed based on Eq. 2 and results are shown in Fig. 5a.

Table 1. Fitting model used to estimate a surface for the bottom fibers of the bridge's girders [*]

Models	Equation of models
$S_{r1} = f(x, y)$	$ax + b$
$S_{r2} = f(x, y)$	$ax^2 + bx + c$
$S_{r3} = f(x, y)$	$ax + by + c$
$S_{r4} = f(x, y)$	$ax^2 + bx + cy + d$

[*] x and y here are coordinates of the point cloud in a global coordinate system

Table 2. Errors of the best fitting surface for the bottom fibers of the bridge's girders

Fitting surface models	RMSE (mm)						
	Girder						Mean
	1	2	3	4	5	6	
S_{r1}	6.4	4.3	5.4	4.3	6.4	5.0	5.3
S_{r2}	6.3	4.3	5.4	4.3	6.4	5.0	5.3
S_{r3}	6.2	4.3	5.2	4.3	6.4	5.0	5.2
S_{r4}	2.0	2.0	1.8	2.4	4.1	2.8	2.5

Table 3. Coefficients of the fitting surface, Sr4 for each girder

| Girder	Coefficients of S_{r4}			
	a	b	c	d
1	-0.0003	0.6421	-0.0064	-308.9907
2	-0.0003	0.6353	-0.0033	-309.0302
3	-0.0003	0.6161	0.0056	-309.0445
4	-0.0002	0.3305	-0.0021	-159.1381
5	-0.0003	0.6333	-0.0027	-309.0141
6	-0.0002	0.3203	-0.0067	-149.0161

In the P2C and C2C methods, the cell grid was employed to decompose an initial 2D bounding box of both reference and sampling data sets into uniform cell grids, in which the cell size is of key importance. However, this value is often empirically selected. As such, cell sizes of 0.1m (C1), 0.2m (C2) and 0.4m (C3) were used to investigate the impact of the cell size on the results. Moreover, a robust PCA method (Laefer et al., 2017) was employed to estimate a normal of $S_{r,local}$ in the P2C method and of $S_{r,ij}$ and $S_{s,ij}$ in the C2C method (Fig.s 3 and 4). Then, the vertical clearance was estimated based on Eq. 2. Results are shown in Fig. 5b-d for the P2C method and Fig.s 5e-g for the C2C method.

Results show consistence between contours of vertical bridge clearance values derived from the proposed methods (Table 4 and Fig. 5). Particularly, in this case study, the vertical displacements from the P2S method slightly differ from those from the P2C method, even when a cell size of 0.4m is used in the P2C method. Morever, the average of the vertical clearance values from the C2C method is slightly smaller than those from the other methods, and this mean is about 3.2mm (P2S vs. C2C_C1) (Table 3). Although the maximum and minimum of the results from the 3 methods differ up to 8.9mm (PS2 vs. P2C_C2) and

16mm (P2S vs. C2C_C3), with a 95% confidence interval, bounds of the vertical clearance values are less than 1.0mm (P2S vs. P2C) and 9.3mm (P2S vs. C2C_C3).

Table 4. Summarized vertical clearance values from the proposed methods

Statistical quantity (m)	Methods						
	P2S	P2C			C2C		
		C1	C2	C3	C1	C2	C3
Mean	5.560	5.560	5.560	5.560	5.556	5.558	5.558
Std.	0.091	0.091	0.091	0.091	0.106	0.107	0.108
Min.	5.328	5.327	5.327	5.328	5.328	5.334	5.337
Max.	5.886	5.878	5.877	5.878	5.876	5.874	5.870
Bounds	5.559/	5.559/	5.559/	5.559/	5.554/	5.553/	5.550/
	5.560	5.560	5.560	5.560	5.559	5.562	5.566
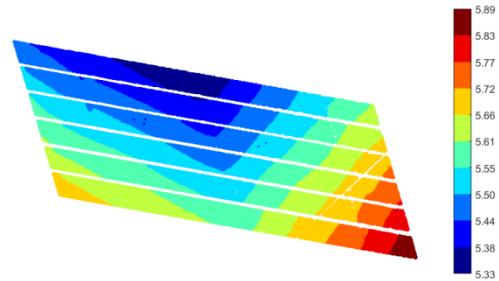
a) P2S
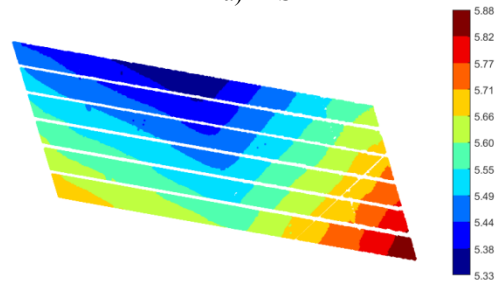
b) P2C with a cell size of 0.1m
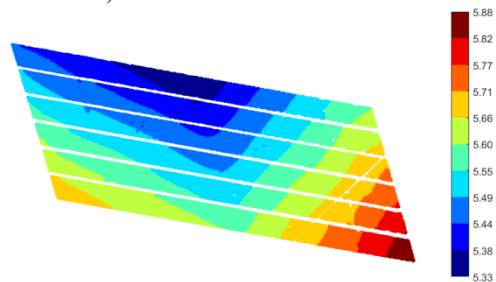
c) P2C with a cell size of 0.2m

d) P2C with a cell size of 0.4m

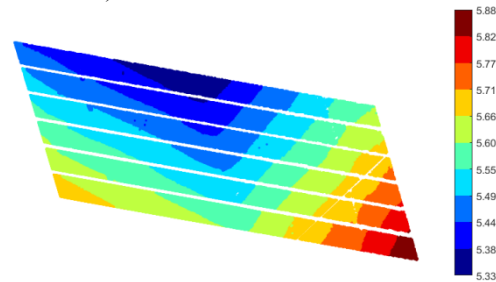e) C2C with a cell size of 0.1m

f) C2C with a cell size of 0.2m
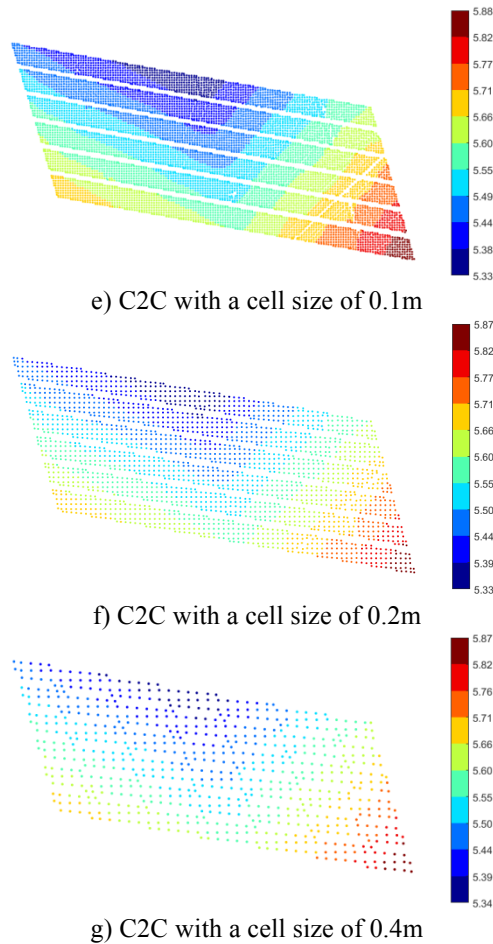

g) C2C with a cell size of 0.4m

Figure 5. Vertical clearance values of the bridge as obtained by the proposed methods (Unit in meter)

This case study shows that these consistently vertical clearance values can be obtained by the three proposed methods. However, when using the P2S method, it is a challenge to determine the best fit model of the reference surface since the close-form of this surface is unknown. Particularly, when a structure is in service and subject to damage, describing a best fit surface as a smooth surface may not describe accurately the reference surface because local damage/deformation may exist. In addition, the P2C and C2C methods provide straightforward procedures to estimate vertical clearance. However, a challenge for these methods is the selection of the cell size, such that the local surface can represent accurately the reference surface at a specific location. In theory, the cell size could adapt to the local curvature of the surface, in which the cell size is small when the surface has a small curvature, and vice versa. Thus, in practice, the P2C and C2C methods are recommended since they are simple methods and do not require heavy computation for a large data set.

Another important issue is to select reference data or a reference surface from the data sets because the accuracy of the fitting surface representing the reference surface is also one of the key impacts when estimating vertical clearance. For example, when the road surface in this case study is chosen as reference surface while the bottom surfaces of a bridge's girders is fixed as the sampling surface, results may change. To test this, a similar procedure as above was used for the

3 methods (P2S, P2C and C2C), where the cell size of 0.2m was used for the P2C and C2C methods. In the P2S method, the best fit surface obtained is $S_r = -0.0026x^2 + 5.238y - 0.041y - 2568$, with a RMSE by 23.9mm. Resulting vertical clearance values are shown in Fig. 6.

a) from the P2S method

b) from the P2C method
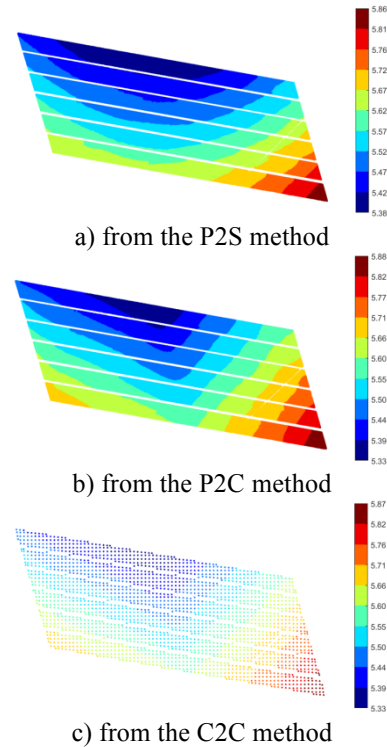

c) from the C2C method

Figure 6. Vertical clearance values from the proposed methods when the road surface is used as a reference surface (Unit in meter)

When the road surface is considered as the reference surface, patterns of vertical clearance values from the P2S method differ from the ones derived when using the bottom surface of the bridges girders as the reference surface (Fig. 5a vs. Fig. 6a). With a 95% confident interval, the difference between the two vertical clearance estimations can reach 47.5mm, in which the lower bounds of the vertical clearance in this case was 5.564m. On the other hand, the vertical clearance estimations resulting from the P2C and C2C methods, show no significant difference when the reference surface is changed: the maximum difference is 5.1mm for the mean value of the vertical clearance derived from the P2C method (Fig. 6b and c). Therefore, it is roughly concluded that the selection of the reference surface strongly impacts the estimated vertical deformation when using the P2S method while the other two methods are only slightly affected.

V. CONCLUSIONS

Measuring deformation of structures is a key factor in structural assessment. Contact methods can produce high accuracy results; however, the methods also have several limitations, for example measuring only at

specific location of the structure and high costs. Laser scanning enables capture entire surfaces of the structure accurately and efficiently. That offers an alternative approach to measure deformation of the structure. This paper proposed and evaluated three processing methods, called point-to-surface (P2S), point-to-cell (P2C) and cell-to-cell (C2C) to estimate the deformation of a structure based on laser scanning data. Through a case study of bridge vertical clearance estimations, where the bottom surface of a bridge's girders was considered the reference surface, is shown that slight differences occur between those methods. The difference in the mean estimated value of the vertical clearance is no more than 3.2mm (P2S vs. C2C_C1). Interestingly, in this case study, for the P2S method, the cell size seemly does not affect the vertical clearance of the bridge. On the other hand, it is shown that the P2S method is sensitive to the choice of reference surface or best fiting surface However, the choice of reference surface does not seem to impact both the P2C and C2C. As a conclusion, the P2C and C2C methods are recommend to measure deformation of the structure, particularly in case of large point clouds. Reason is these methods are simple, low-cost, and do not require prior knowledge on a close-form of the reference surface. However, these methods require a predefined cell size, which could be selected based on the curvature of the reference surface.

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